

# ASTM E1592 TEST RESULTS SL1500 PANEL

Zimmerman Metals, Inc. Over 60 years of Zuality Workmanship and Service

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January 22, 1997

Engineering Report 96505-1

Zimmerman Metals, Inc., SL1500 Snap Lock Panel Uplift Test

#### SUMMARY

Beginning November 19, 1996, testing of Zimmerman Metals, Inc. SL1500 Snap Lock roof panels was performed to determine their loading characteristics under uniform static uplift loads. The panels were 16 inches wide with a nominal 1 1/2 inch Snap Lock rib, 24 gage steel and were tested with spans of 3 feet, 6 inches and one foot. The panels were tested in accordance with ASTM E 1592, "Standard Test Method for Structural Performance of Sheet Metal Roof and Siding Systems by Uniform Static Air Pressure Difference." The 3-foot, 6-inch span test specimen failed at an uplift load of 31.2 PSF when a seam separated. The 1 foot span test specimen failed similarly at a load of 29.1 PSF.

Respectively submitted,

Arthur C. Ivey, P.E.

Todd Breedlove

Senior Laboratory Technician



#### INTRODUCTION

On November 19, 1996, testing of SL1500 Snap Lock Panels manufactured by Zimmerman Metals, Inc. was initiated to determine their loading characteristics under uplift loading conditions. The panels were tested in accordance with ASTM E-1592, "Structural Performance of Sheet Metal Roof and Siding Systems by Uniform Static Air Pressure Difference." The panels were constructed on the test fixture by Architectural Metal Specialties, Inc. personnel. The panels were tested to failure. The loading characteristics were recorded and photographed. Color photographs of the test specimen and failure modes are included in this report.

### SPECIMEN

The specimen tested was an assembly of 16-inch SL1500 Snap Lock panels supplied by Zimmerman Metals. The panels were installed on a support structure using the appropriate formed sections, clips, and fasteners.

The SL1500 Snap Lock panels covered a nominal 16 inches and had 1 1/2-inch ribs(Figure 1). The material used to form the panels was 24 gage, 56.6 KSI average yield strength (determined by Cerny & Ivey, see attached Laboratory Report 96505) cold formed painted galvanized sheet steel.

The supports used for testing were 16-gage cee and zee sections. The eave end was supported by an 8-inch deep cee section. The remaining supports were 8-inch deep zee sections. The supports were made of cold formed steel and were spaced at 3 feet, 6 inches on centers for the first test and 1 foot on centers for the second test.

Clips supplied by Architectural Metal Specialties (11/2 inch 24 ga. steel Snap Lock clips) were used to attach the panels to the formed cee and zee sections (Photograph A).

Each clip was attached to the support with two 1 inch by No.8 self drill and tap pancake head screws (Photographs B through D). The starting edge of the first panel was secured with a 24 gage J-track. The J-track was attached to supports at 1 foot on centers. A formed edge detail lapped over the J-track and panel leg (Photographs E and F). The 24 gage edge detail was attached to the J-track with 1 inch by No. 8 self drill and tap pancake head screws spaced 1 foot on centers.

During installation each panel was lapped over the preceding panel and locked into place by hand (Photograph G). A panel was split to finish the field edge. No additional fasteners were installed on the field edge. The final specimen size was 21 feet, 0 inches long (11 feet, 0 inches for 1 foot on centers) by 6 feet, 8 inches (five panels) wide

(Photographs H through I).

### PROCEDURE

The panels were assembled in the test chamber by personnel from Architectural Metal Specialties, Inc. (Figure 2). A plastic film (6 mil thickness) was placed by Cerny & Ivey Engineers, Inc. personnel during panel installation to seal the panel assembly against air leakage. The plastic film was pleated so that it did not affect the test results by causing fillets or distribution of the pressure by bridging across members. The panel edges and plastic sheet were then sealed to the edge of the test chamber.

Deflection gages, accurate to 0.0005 inch, were installed above the ribs on the edge of the third panel at the center of the center span (two gages) and over the adjacent support location (two gages). Two gages were placed on the center of the third panel flat section at these same lines. The chamber pressure was measured at both ends (diagonal corners) of the chamber using water manometers accurate to 0.01 inch of water.

Pressure was applied to the specimen to a reference "zero" load of 1-inch of water (5.2 PSF) and maintained for 60 seconds; the deflection gages were then read. The load was then reduced to zero for a recovery period before increasing

to the next load increment, where it was maintained for 60 seconds; deflections were then read. The load was returned to an actual zero load for a recovery period and then returned to the reference "zero" load and maintained for 60 seconds; set deflections were then read. This procedure was repeated until failure of the panel occurred. This procedure was performed in accordance with ASTM E-1592, "Structural Performance of Sheet Metal Roof and Siding Systems by Uniform Static Air Pressure Difference."

### RESULTS

Test No. 96505-1 (Photographs J through M)

Specimen: Zimmerman Metals 1 1/2-inch Snap Lock panel,

24 gage, 56.6 KSI Painted galvanized steel,

3 1/2 foot supports

Date: November 19, 1996

	RIB DEFLECTION (INCHES)					RIB SET (INCHES)						
LOAD	****	MIDSPA	.N	********	SUPPORT			MIDSPAN	J	<del></del>	SUPPOR'	Γ
(PSF)	_4_	6	AVG	_1_	3	AVG	_4_	_6_	AVG	_1_	_3_	AVG
5.2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7.8	0.01	0.00	0.00	0.01	0.00	0.00	0.01	0.00	0.00	0.01	0.00	0.00
10.4	0.04	0.05	0.04	0.03	0.03	0.03	0.02	0.01	0.02	0.01	0.01	0.01
13.0	0.06	0.07	0.06	0.04	0.05	0.04	0.03	0.02	0.02	0.02	0.02	0.02
15.6	0.10	0.10	0.10	0.08	0.07	0.08	0.03	0.02	0.02	0.02	0.02	0.02
18.2	0.13	0.12	0.12	0.10	0.08	0.09	0.04	0.03	0.04	0.03	0.02	0.02
20.8	0.17	0.16	0.16	0.13	0.12	0.12	0.05	0.04	0.04	0.04	0.04	0.04
26.0	0.26	0.23	0.24	0.20	0.18	0.19	0.09	0.08	0.08	0.07	0.07	0.07
31.2	0.53	0.47	0.50	0.43	0.40	0.42						

	MIDPANEL DEFLECT	ION (INCHES)	MIDPANEL SET	(INCHES)
LOAD	MIDSPAN	SUPPORT	MIDSPAN	SUPPORT
(PSF)	5	2	5	2
5.2	0.00	0.00	0.00	0.00
7.8	0.00	0.30	0.01	0.00
10.4	1.04	0.96	0.02	0.00
13.0	1.11	1.05	0.03	0.00
15.6	1.26	1.19	0.03	0.01
18.2	1.40	1.33	0.03	0.01
20.8	1.57	1.51	0.04	0.02
26.0	1.84	1.79	0.07	0.04
31.2	2.34	2.31		

### **OBSERVATIONS**

7.8	PSF	Visible	pan	deflection
,	101	^ TO TO TC	Dan	- ベニエニとしてエレロ

<sup>5.2</sup> PSF

Panels return to initial shape Increased pan deflection, visible spreading 18.2 PSF of ribs

Panels return to initial shape 5.2 PSF

<sup>20.8</sup> PSF Ribs appear to begin separating
5.2 PSF Panels return to initial shape
31.2 PSF First panel rib unseams to midpoint

Test No. 96505-8 (Photographs N through O)
Specimen: Zimmerman Metals 1 1/2-inch Snap Lock panel,
24 gage, 56.6 KSI Painted galvanized steel,

1 foot supports November 22, 1996 Date:

	RIB DEFLECTION (INCHES)						RIB SET (INCHES)					
LOAD	MIDSPAN			SUPPORT		MIDSPAN			SUPPORT			
(PSF)	_4_	_6_	AVG	_1_	_3_	_AVG_	_4_	_6_	AVG	_1_	_3_	AVG
5.2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7.8	0.01	-0.01	0.00	0.01	0.00	0.00	0.01	-0.01	0.00	0.01	0.00	0.00
10.4	0.01	0.00	0.00	0.01	0.00	0.00	0.01	0.00	0.00	0.01	0.00	0.00
13.0	0.01	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
15.6	0.02	0.02	0.02	0.02	0.02	0.02	0.01	0.01	0.01	0.00	0.01	0.00
20.8	0.05	0.04	0.04	0.05	0.04	0.04	0.01	0.00	0.00	0.01	0.01	0.01
26.0	0.07	0.06	0.06	0.07	0.06	0.06	0.03	0.01	0.02	0.02	0.01	0.02

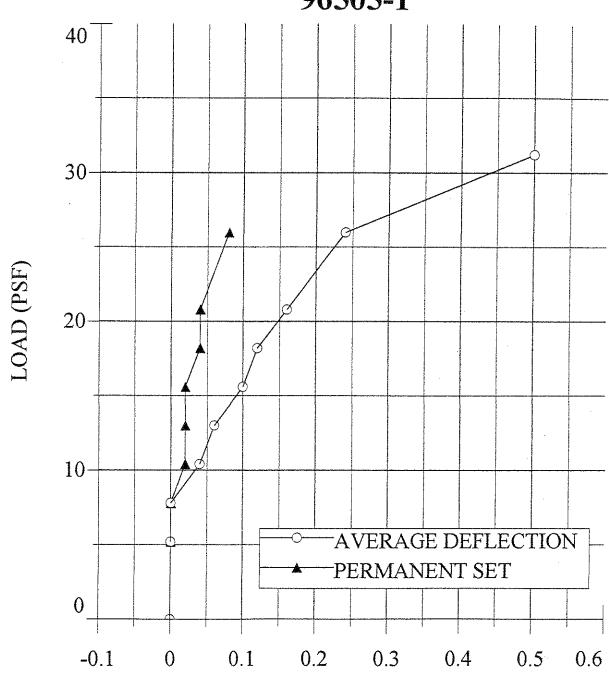
	MIDPANEL DEFLECTION	(INCHES)	MIDPANEL SET	(INCHES)
LOAD	MIDSPAN	SUPPORT	MIDSPAN	SUPPORT
(PSF)	5	2	5	2
5.2	0.00	0.00	0.00	0.00
7.8	0.07	0.06	-0.03	0.03
10.4	0.90	0.91	-0.03	0.10
13.0	1.09	1.09	-0.03	0.04
15.6	1.23	1.22	0.01	0.04
20.8	1.44	1.42	0.01	0.03
26.0	1.58	1.56	0.02	0.03

### **OBSERVATIONS**

	PSF PSF	Visible pan deflection Panels return to initial shape
13.0	PSF	Increased pan deflection. Visible spreading
		of ribs
5.2	PSF	Panels return to initial shape
15.6	PSF	Increased pan deflection and rib spread
5.2	PSF	Audible sound as ribs rotate during return
20.8	PSF	2-3 seam deflects noticeably
5.2	PSF	2-3 seam does not return fully
29.1	PSF	2-3 seam separates

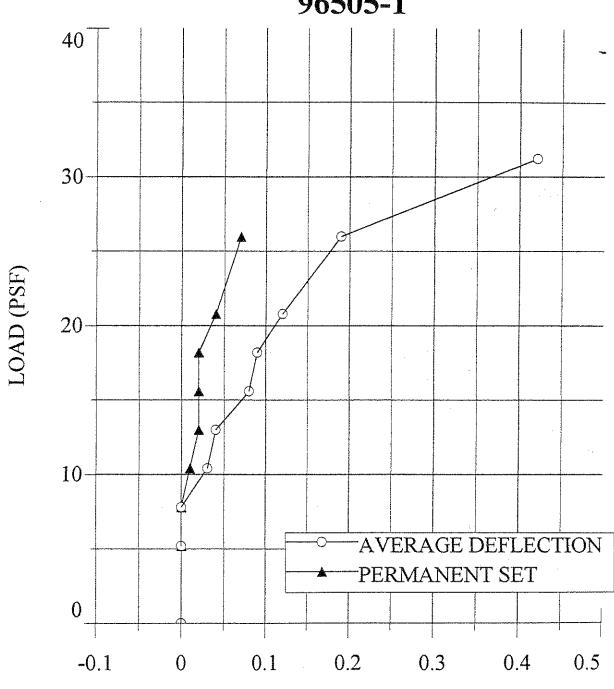
The deflection of the panels during testing is represented graphically in the pages that follow:





## RIB MIDSPAN

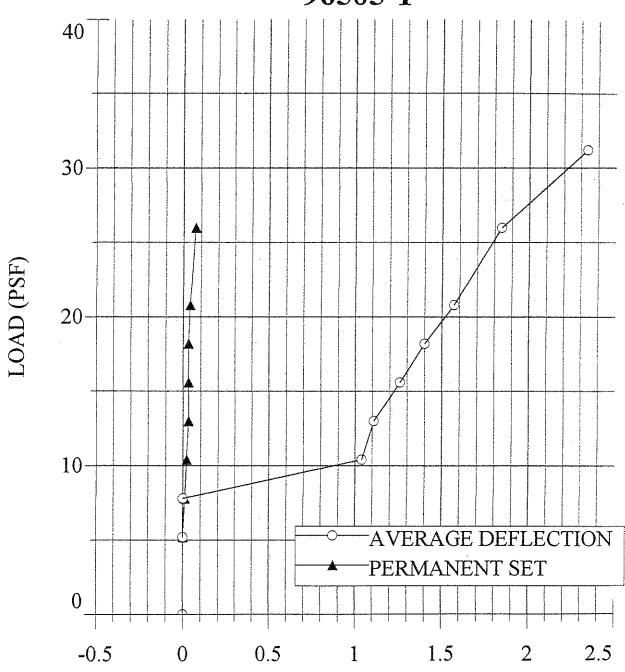
## **PANEL TEST** 96505-1



**DEFLECTION (INCHES)** 

RIB SUPPORT

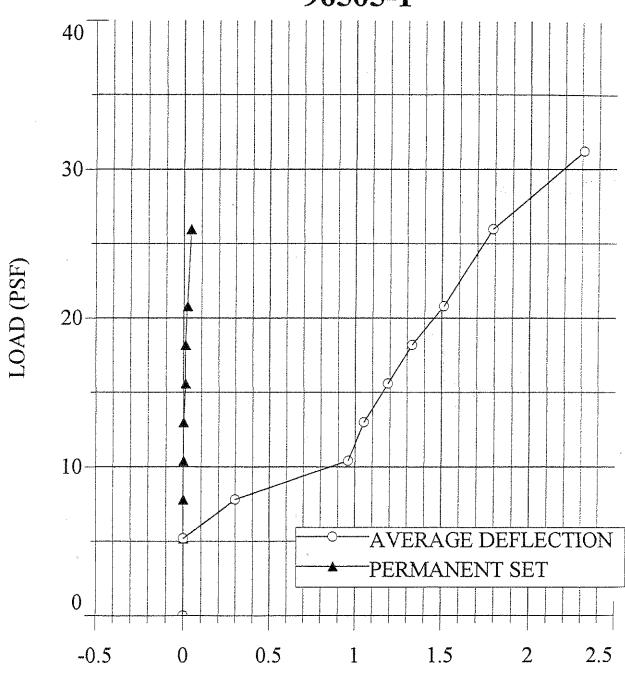
## **PANEL TEST** 96505-1



DEFLECTION (INCHES)

## MID PANEL MIDSPAN

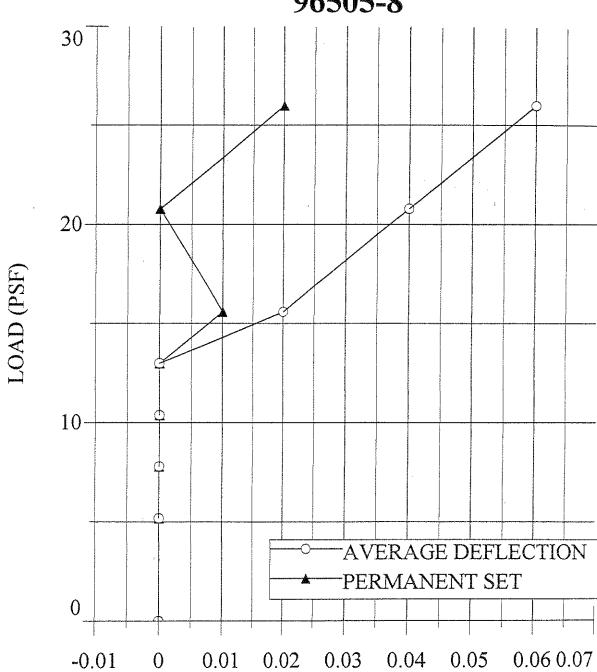
## **PANEL TEST** 96505-1



**DEFLECTION (INCHES)** 

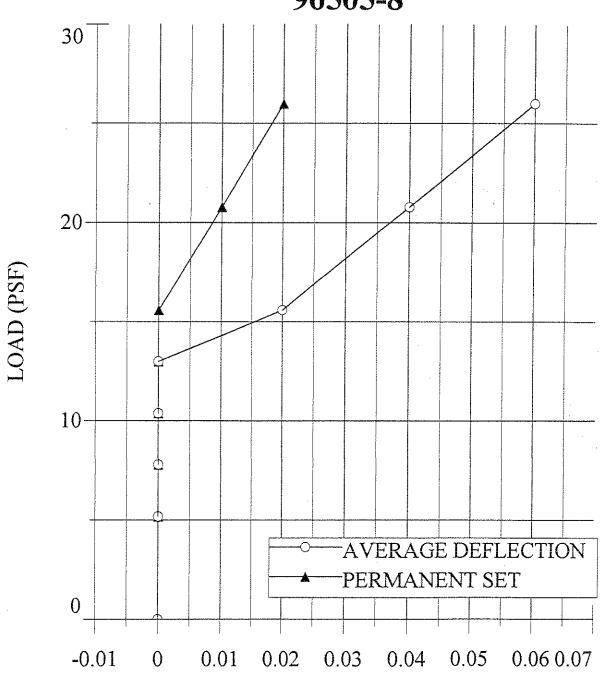
### MID PANEL SUPPORT





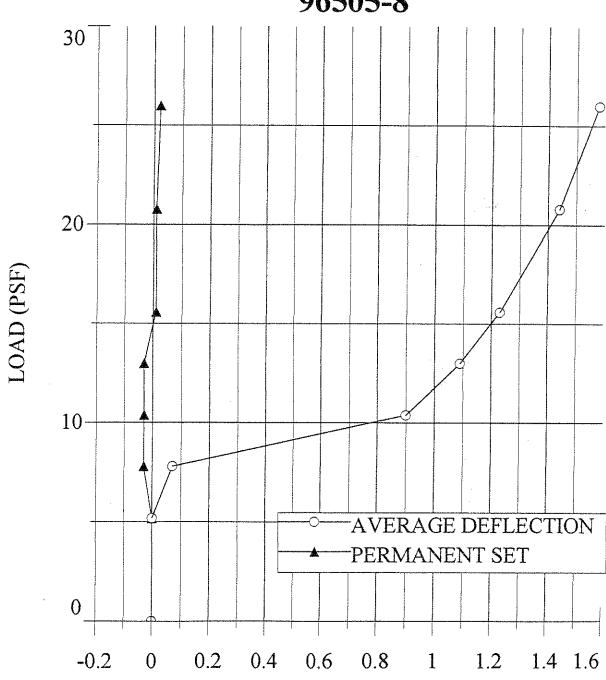
## RIB MIDSPAN





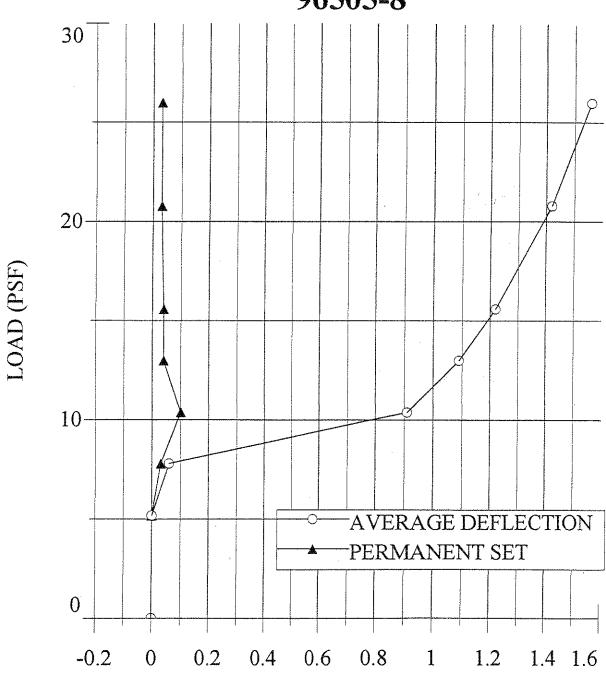
RIB SUPPORT





## MID PANEL MIDSPAN





## MID PANEL SUPPORT

### CONCLUSIONS

The allowable panel load for wind was calculated using the test results and factors of safety prescribed by section F1 of the "Cold Formed Steel Design Manual" 1986 edition with 1989 addendum American Iron and Steel Institute. The allowable load calculation was based on the following equation:

(Eq F1-4)  $R \ge (2.5D + 2.5W)/1.333$  for connection failure where

R = required panel strength based on tests

D = dead load (included in test data)

W = wind load

The required connection strength was divided by 1 1/3 for wind loading.

Based on this equation, the allowable panel loads were calculated as follows:

### 3 Foot - 6 Inch Span

$$W = (1.333(R))/2.5$$

or

$$W = (1.33(31.2))/2.5 = 16.6 PSF$$

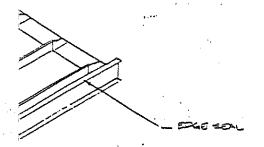
### 1 Foot Span

$$W = (1.333(31.2))/2.5 = 15.5 PSF$$

Comparison of these values the load carrying capacity of the panel system decreased for the smaller panel span due to the presence of the panel clip in the Snap-Lock seam. The minimum allowable wind load for panel spans (clip spacing) between 1-foot and 3-foot-6 inches is 15.5 PSF. The recommended clip spacing in this span range is 3-foot-6 inches which will give an allowable wind load of 16.6.

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TEST SPECIMEN

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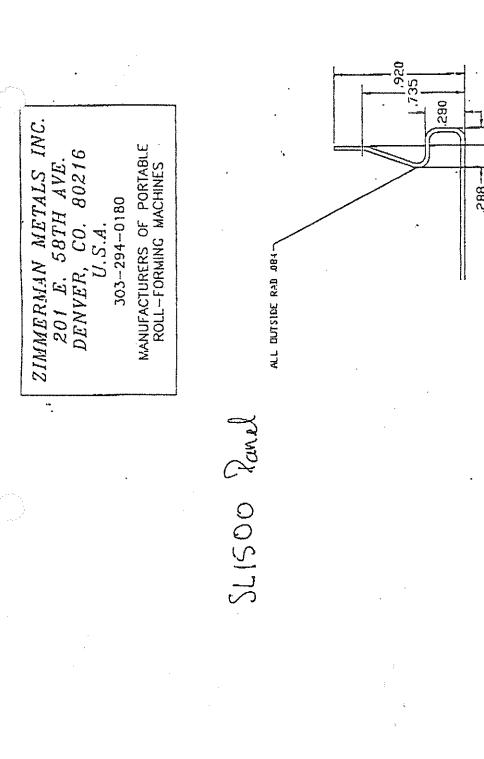
IS IN ACCORDANCE WITH ASTM E-1592, THOD FOR STRUCTURAL PERFORMANCE OF AND SIDING SYSTEMS BY UNIFORM STATIC ERENCE" AS OUTLINED IN THE U.S. ARMY GUIDE SPECIFICATION FOR MILITARY -07416 (SSSMR SYSTEMS).

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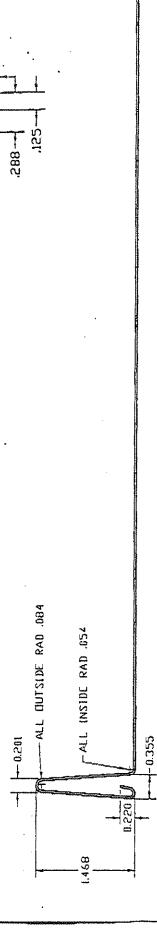


Figure 1

### LABORATORY REPORT

NUMBER: 96505

January 22, 1997

REFERENCE

TEST DESCRIPTION

Tensile Test

5650 PEACHTREE PARKWAY (404)449-6936 NORCROSS (ATL) GA 30092 SAMPLE(S) RECEIVED IN LABORATORY

DATE: 11/19/96

CTB BY:

CERNY & IVEY ENGINEERS, INC.

CONSULTING ENGINEERS - TESTING LABORATORY

VIA: AMSI

SAMPLE DESCRIPTION

Steel sheet samples from tested Panels: 1 1/2 Snap Lock SL1500 1 inch Snap Lock SL1000 Nail-Leg NS1000 Standing Seam SS1500

Mr. Eric Paulsen Zimmerman Metals, Inc. 201 East 58th Avenue Denver, CO 80216

### INTRODUCTION

To determine the strength of the materials used to fabricate the panels used in uplift testing, samples were randomly removed for tensile testing. The selected samples were machined in accordance with ASTM A370 and tested per ASTM E8. The material yield strength and ultimate strength were calculated from the test results.

### **RESULTS**

<u>Sample</u>	Base Metal Thickness(in)	% Elongation	Yield Strength(KSI)	Ultimate Strength (KSI)
SL1500A SL1500B SL1500C AVERAGE	0.025 0.025 0.025	20.00 20.45 21.80 20.75	55.3 56.9 57.7 56.6	60.3 58.4 59.4 59.4
SL1000A SL1000B SL1000C AVERAGE	0.025 0.025 0.025	20.70 20.05 18.75 19.83	62.7 62.4 62.7 62.6	63.7 63.6 63.9 63.7
NS 1000A NS 1000B NS 1000C AVERAGE	0.024 0.024 0.024	21.60 19.40 18.15 19.72	62.5 60.8 64.4 62.6	65.3 66.6 66.3 66.1
SS 1500A SS 1500B SS 1500C AVERAGE	0.023 0.023 0.023	21.50 21.75 23.00 22.08	61.0 63.3 63.6 62.6	65.7 66.3 66.3 66.1
SS 1500D SS 1500E SS 1500F AVERAGE	0.024 0.024 0.024	23.8 22.2 23.6 23.2	56.1 56.1 56.1 56.1	66.7 67.0 66.2 66.6

NOTE: THE SAMPLES AND/OR ITEMS PROVIDED FOR ANALYSIS WILL BE DISCARDED THIRTY (30) DAYS AFTER DATE OF THIS REPORT UNLESS FURTHER NOTIFICATION IS RECEIVED.

If we can be of further service in this matter, please do not hesitate to call.

Arthur C. Ivey, P.E.

Respectfully submitted,

Todd Breedlove Senior Laboratory Technician